



Bulletins

Glulam-Bulletin

December 2014

General

Load bearing components made from glued laminated timber (Glulam) are designed and carefully manufactured, top-quality construction components made from an improved material. In spite of the fact that a harmonized European product standard EN 14080: 2005 exists and the European design standard EN 1995-1-1 is to be used in Germany starting from 1.7.2012, Glulam to be used in Germany still has to comply with DIN 1052: 2008, Annex H. Glulam therefore may only be manufactured by companies who have a corresponding certification concerning the gluing suitability of load bearing timber construction components according to German standard DIN 1052: 2008.

A list of companies who have provided this certification can be found on the website www.brettschichtholz.de In the following a few generally accepted, material dependent rules shall be explained, adherence to which assures long-term stability of the structure and preservation of the appearance.

Glulam without an Ü-Sign is currently not applicable in Germany

In addition a few important terms and definitions for the quality determination of Glulam will be explained.

Strength classes

Glulam is, according to DIN 1052: 2008, produced and divided into strength classes. The strength classes and correlation to the previous strength classes according to DIN 1052-1/ A1: 1996-10 can be taken from table 1. The numerical value of the Glulam classes stands for the characteristic value of the bending strength (for Glulam according to DIN 1052: 2008) in N/mm². The „h“ and/or „c“ with the designation of DIN 1052: 2008-12 stands for homogeneous and/or combined symmetrical built up Glulam. A classification to a “combination” strength class GL XX c can be achieved by the manufacturer of the glued laminated timber by means of various cross sectional layouts. Glulam of higher strength classes can be exceedingly economically manufactured with a combined layout since the higher strength boards accumulated in a grading process can then be directed to the areas of greater tensile loads and the boards of lower strength to the core or the compression loads. Due to the high costs and greater lead times required, homogeneous glued laminated timber with strength class greater than GL 24 should only be used in exceptional cases, e.g. for components stressed primarily by normal forces.

The following standard qualities are available: GL 24c, GL 24h, GL 28c, and GL32c. If the strength class is not specified, GL 24c is delivered.

With the ordering of GL 32c or h, in addition, one has to bear in mind that not all grading machines required for the manufacturing allow for the grading of species other than spruce/fir.

Table 1
Strength classes

DIN 1052: 2008	Previous, no longer applicable, designations according to: DIN 1052-1/A1: 1996
GL 24c, h	BS 11
GL 28c	BS 14
GL 32c	BS 16

Cross sectional layout of members of various heights

Large volume, order related Glulam construction members manufacturing of a higher strength class than GL 24 have, as a rule, a combined layout. At the point of maximum bending stress M/W, the layout required for the respective strength class must be given. The reduction of the proportionate height of a boundary area with higher strength lamellae in the direction of the support is regarded as structurally unquestionable.

Reinforcements for tension perpendicular to the grain

DIN 1052: 2008 and DIN EN 1995-1-1 (Eurocode 5-1-1): 2010 permit the design of members subjected to tension stresses perpendicular to the grain either with or without reinforcements. Designs with reinforcements for tension stresses parallel to the grain according to DIN 1052: 2008 or DIN EN 1995-1-1/NA: 2012 (National Annex to Eurocode 5-1-1) are recommended exclusively for pitched cambered beams.

Labelling

Components made from Glulam comply with the building authorities requirements. They are labelled by the manufacturer with the German compliance mark (Ü-mark). At the time of this bulletin’s printing, Glulam labelled only with the CE mark according to DIN EN 14080: 2005 is not applicable in Germany.

Surface protection

To avoid non-beneficial moisture absorption during transportation and assembly as well as for improving the cleanability, the surfaces, with larger construction components and also the cross-grained wood, should be provided with a suitable temporary weather protection coating.

Surface qualities

Glulam components can be manufactured with various surface qualities and thus fulfil variable creative requirements. The desired surface characteristics are, in each case, contractually agreed upon and are, for example, itemised in the technical specifications. If nothing else is stipulated visible quality is applied.

Transport and Assembly

Transport and assembly of Glulam components should categorically only be carried out by experienced and therefore fully equipped certified specialists. Thereby, among other things, the following is to be observed:

- Sufficient bracing, also whilst under construction.
- Soiling prevention.
- During the lifting process, as a rule, the entire section should be encompassed with heavy duty binding or some other suitable device.
- Duly storage on site. Attention should be paid to the fact that coverings for transport have to be removed in order to avoid condensate and blue stain. The members should be protected by suitable coverings against moistening and contamination subsequently.
- Suitable protection of edges.
- Precise axial orientation of glulam members and temporarily bracings until the bracing is mounted.
- Final alignment of construction.
- Protection against corrosion should be carried out before the steel elements are mounted in order to avoid contamination of glulam surfaces by rust.
- If steel is cut or welded on site the nearby glulam surfaces should be covered in order to avoid contamination of the glulam surfaces by discolouration and rust.

Table 2
Surface qualities of Glulam

Criteria ¹	Industrial quality	Visible quality	Selection quality
1 Firmly grown knots ^{2,3}	Permitted	Permitted	Permitted
2 Fallen and loose knots ^{2,3}	Permitted	Up to $\varnothing < 20$ mm ⁴ permitted from $\varnothing > 20$ mm to be replaced in the works ⁴	To be replaced in the works
3 Resin gall ^{3,5}	Permitted	Resin galls are permitted up to 5 mm	Resin galls are permitted up to 3 mm
4 Knots and faulty points improved by means of knot hole plugs or „ships“ ³	Not necessary	Permitted	Permitted
5 Knots and resin galls improved by means of filler compound ³	Not necessary	Permitted ⁶	Permitted ⁶
6 Insect attack ³	Permitted are burrows up to 2 mm	Permitted are burrows up to 2 mm	Not permitted
7 Pith	Permitted	Permitted	Pith visibly on the outer lamellas is not permitted
8 Cracks caused by shrinking ^{3,5,7}	No limit	Up to 4 mm	Up to 3 mm
9 Discolorations as a result of blue stain, rote und braune nagelfeste Streifen ⁵	No limit	Up to 10 % of the visible surface of the whole construction component	Not permitted
10 Mould ⁵	Not permitted	Not permitted	Not permitted
11 Contamination of the surface ⁵	Permitted	Not permitted	Not permitted
12 Distance between fingerjoints	No limit	No limit	On visible remaining outer lamellas, the distance between one another has to be at least 1 m
13 Surface	Levelled out	Planned and camfered chatter permitted up to a depth of 1 mm	Planned and camfered chatter permitted up to a depth 0,5 mm

1 Deviations from the limit values defined in the following in lines 2, 3, 6-9, 12, 13 are to be tolerated to the following extent: maximum of three deviations/m² visible surface for the quality of vision, maximum of one deviation/m² visible surface for selection quality.
2 Permissible knot size according to DIN 4074: 2012
3 Without limitation of the number
4 Measurement of the diameter of the knots analogue to the measurement of the diameter of individual knots for scantlings according to DIN 4074-1: 2012, 5.1.2.1.

5 Delivery condition
6 Filler compound which can be painted over is to be demanded explicitly.
7 As in all constructional solid wood products cracks can be present. The depth of the crack, measured with a 0.1 mm thick feeler gauge and independent of the quality of the surface for construction components, may be, for members not being subject to tensile-stresses perpendicular to the grain up to 1/6 of the width of the construction component, for members being subject to tensile-stresses

perpendicular to the grain, up to 1/8 of the width of the component from each side. With deeper cracks the non-critical state should be checked by an expert.

A more comprehensive and illustrated description of the surface qualities can be found in the article RADOVIC/WIEGAND „Oberflächenqualität von Brettschichtholz“ [Surface quality of glued laminated timber, German language only], which is located in the download area of the website www.brettschichtholz.de

Table 3
Maximum permitted deviations

		Maximum permitted deviations	
		Straight members	Curved members
Cross-sectional width	for all widths	± 2 mm	
Cross-sectional height	h ≤ 400 mm h > 400 mm	+ 4 mm to - 2 mm + 1 % to - 0,5 %	
Maximum deviation of the angles of the cross section from the right angle		1:50	
Length of a straight member or developed length of a curved member	l ≤ 2 m 2 m ≤ l ≤ 20 m l > 20 m	± 2 mm ± 0,1 % ± 20 mm	
Longitudinal warping measured as the maximum gauge over a length of 2 000 mm without consideration of precamber		4 mm	—
Deviations of the gauge of curved members per developed length in m	≤ 6 Laminations > 6 Laminations	— —	± 4 mm ± 2 mm

Deviation in sizes

Deviations in sizes determined by DIN EN 390: 1995 for a reference moisture content of 12% must not exceed the values listed in Table 3. In addition to the specifications of DIN EN 390: 1995, Table 3 also contains maximum permitted deviations for curved members. A curved member here is a member with a precamber of more than one hundredth of the span.

Limiting values for deflections

As in the former national standard DIN 1052: 2008 the limiting values for deflections given in DIN EN 1995-1-1: 2010 and the respective National Annex DIN EN 1995-1-1/NA are only recommendations! Clients and designers have to agree upon the limiting values for deflections for each specific building. On the basis of long-term experience the Studiengemeinschaft Holzleimbau e.V. recommends the following values (values in brackets apply for cantilevered members).

Table 4
Recommended limiting values for deflections for beams

	w_{inst}	$w_{net,fin}^{1)}$	w_{fin}
Members other than members according to line 2	$l/300$ $l/150^{2)}$	$l/300$ $l/150^{2)}$	$l/200$ $l/100^{2)}$
Precambered members or members of minor importance such as members for agricultural buildings, rafters and purlins	$l/200$ $l/100^{2)}$	$l/250$ $l/125^{2)}$	$l/150$ $l/75^{2)}$

¹⁾ Deviating from DIN EN 1995-1-1: 2010 but in accordance with DIN EN 1990: 2010 and the future DIN EN 1995-1-1/NA/A1 $w_{net,fin}$ is calculated as follows: $w_{net,fin} = \left(w_{inst,G} + \sum_{i \geq 1} \psi_{2,i} \cdot w_{inst,0,i} \right) (1 + k_{def}) w_c$

²⁾ for cantilevered members

Subsequent block outs, notches, openings, drilled holes and cuttings

In every case they call for a new static analysis.

Structural-physical analysis

Predominantly the outer layers of the Glulam, under construction, absorb moisture. This moisture must be gradually dispersed, until a uniform cross-sectional moisture is achieved. Careful heating and ventilation and the resulting accompanying slow reduction of the relative humidity of air and the corresponding wood moisture serves this purpose.

On the surfaces of the Glulam components and also along the glued joints Shrinkage cracks can appear. As in all constructional timber products cracks can be present. The depth of the crack, measured with a 0.1 mm thick feeler gauge and independent of the quality of the surface for construction components, may be, for members not being subject to tensile-stresses perpendicular to the grain up to 1/6 of the width of the construction component, for members being subject to tensile-stresses perpendicular to the grain, up to 1/8 of the width of the component from each side. With deeper cracks the non-critical state should be checked by an expert.

With direct exposure and strong changing climatic conditions the inclination towards crack formation increases. Even during the planning, protective measures are also provided for the construction in progress. These are, in particular, coverings and backwater free drains.

Priority is given to constructional means of wood protection over preservative treatment. To this belongs, for example, prompt closure of the roof and outer wall surfaces immediately after assembly and also removal of building moisture by ventilation. For reasons of environmental and health protection, constructional means of wood protection measures should be taken that render additional use of preservative treatment unnecessary. According to DIN 68800-1: 2011, in service classes 1 and 2 (moisture content permanently < 20%), no preservative treatment against fungi or insects is required with kiln dried solid wood products such as glulam. Depending on the use class according to DIN 68800-1:2011, you can resort to durable softwood species (with no or limited sapwood) in service class 3. Pine heartwood can be used up to use class 2, larch and Douglas fir heartwood up to use class 3.1. If, in exceptional cases, preservative treatment is required, then products according to a German technical approval applicable to the respective use class shall be used.

Prior to application, however, a written opinion of the preservative agent manufacturer containing the following points should be requested:

- Necessity of surface coating and other moisture protection measures until the members are protected against weathering;
- Compatibility of preservative treatment with surface coating;
- Necessity of pre-treating the surface, in particular in the case of kiln dried spruce wood, to reach the required penetration quantity and, if applicable, penetration depth;
- Fundamental suitability of a surface application with large volume glulam timber components.

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